

# “Movement of Pollutants in Rivers”

## Module 2: Surface Waters, Lecture 2

*Chemical Fate and Transport in the Environment*, 2<sup>nd</sup> edition. H.F. Hemond and E.J. Fechner-Levy. Academic Press. London. 2000.

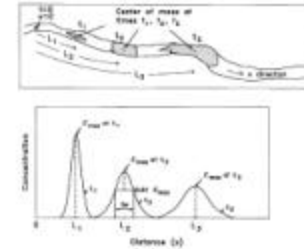
*Principles of Surface Water Quality Modeling and Control*. R.V. Thomann and J.A. Mueller. Harper & Row, New York. 1987.

The velocity of the dye from the injection location to the time 2 location is:

$$V = \frac{L_2}{t_2} = \frac{1025m}{5hr} = 205m/hr$$

## Example Problem 2-2

- The dye concentration profile was measured at time 2 in the dye transport plot, 5 hours after injection. What is the average river velocity if the max. concentration is occurring 1025 m down river from the injection location?



Estimate the longitudinal dispersion coefficient for this river if the standard deviation,  $\sigma_L$ , in the longitudinal direction is approx. 350 m when the chemical has traveled a distance of 1975 m to  $L_3$

The travel time to this location is:

$$t_3 = \frac{L_3}{V} = \frac{1975m}{205m/hr} = 9.6hr$$

The longitudinal dispersion coefficient,  $D_L$ , can then be estimated:

$$D_L = \sigma_L^2 / 2t = (350m)^2 / (2 \cdot 9.6hr) \approx 6400m^2/hr$$

## Estimates of Fickian Transport Coefficients from Flow Data

- Turbulence is caused by velocity shear due to a nonuniform velocity profile. The shear velocity (related to the shear force per unit area) can be estimated:

$$u^* = \sqrt{gdS} \quad \text{Where } d \text{ is the stream depth and } S \text{ is the slope}$$

This shear velocity can be used to estimate the transverse dispersion coefficient,  $D_t$ :

$$D_t \approx 0.15 \cdot d \cdot u^* \quad \text{For straight channels}$$

$$D_t \approx 0.6 \cdot d \cdot u^* \quad \text{For typical natural channels}$$

The following equation can be used to predict the longitudinal dispersion coefficient,  $D_L$ :

$$D_L = \frac{0.011 \cdot V^2 \cdot w^2}{d \cdot u^*}$$

Where  $V$  is the average velocity [L/T]

$w$  is the stream width [L]

$d$  is the stream depth [L]

## Example Design for a Dye Injection Experiment for the Cahaba River

Solve the instantaneous equation for  $M$  to determine the amount of conservative dye to be used:

$$C(x, t) = \frac{M}{2A\sqrt{4pD_L t}} e^{-(x-Vt)^2 / (4D_L t)}$$

- Estimate the average velocity ( $V$ ) and travel time ( $t$ ) to the location of interest ( $x$ ), and determine the corresponding desired dye concentration ( $C$ ) at that location.
- Estimate the longitudinal dispersion coefficient,  $D_L$ .
- Solve for  $M$ , the needed mass of dye to be instantaneously released.

1) Estimate the average velocity ( $V$ ) and travel time ( $t$ ) to the location of interest ( $x$ ), and determine the corresponding desired dye concentration ( $C$ ) at that location.

- The monitoring location is 4 miles from the discharge location ( $x = 4$  miles, 21,120 ft).
- The mean flow for the Cahaba River in this area is 99 MGD, the average width is 30 ft, and the average depth is 1.7 ft.
- The average velocity in this reach is therefore expected to be 3 ft/sec ( $V=3$  ft/sec).
- The travel time is therefore 2 hours (0.08 days)
- The desired dye concentration at the location 4 miles from the discharge location is 250 ppb ( $v/v$ ).

2) Estimate the longitudinal dispersion coefficient,  $D_L$ .

$$D_L = \frac{0.011 \cdot V^2 \cdot w^2}{d \cdot u^*} \quad u^* = \sqrt{gdS}$$

$$g = 32.2 \text{ ft/sec}^2$$

$$d = 1.7 \text{ ft}$$

$$S = 0.01$$

$$V = 3 \text{ ft/sec}$$

$$w = 30 \text{ ft}$$

Therefore,

$$u^* = 0.74 \text{ ft/sec}$$

$$D_L = 71 \text{ ft}^2/\text{sec}$$

3) Solve for M, the needed mass of dye to be instantaneously released.

$$M = \frac{2AC \sqrt{4pD_L t}}{e^{-(x-Vt)^2/(4D_L t)}}$$

$$C = 250 \text{ ppb} = 250/1,000,000,000 = 2.5 \times 10^{-7}$$

$$D_L = 71 \text{ ft}^2/\text{sec}$$

$$t = 7,040 \text{ sec}$$

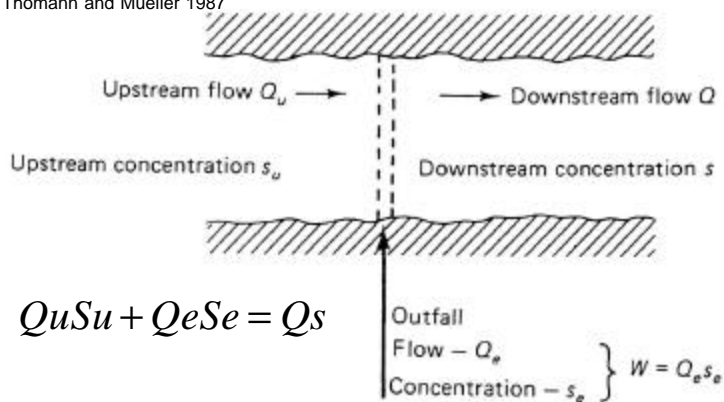
$$x = 21,120 \text{ ft}$$

$$V = 3 \text{ ft/sec}$$

$$A = 51 \text{ ft}^2$$

Therefore,  $M = 0.032 \text{ ft}^3$ , or 0.24 gal (about 1 L)

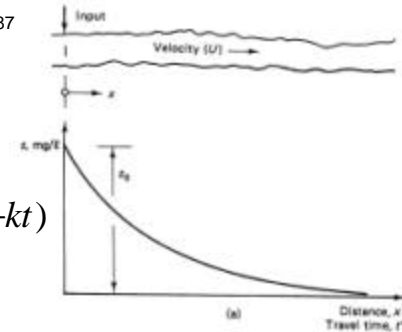
Thomann and Mueller 1987



$$QuSu + QeSe = Qs$$

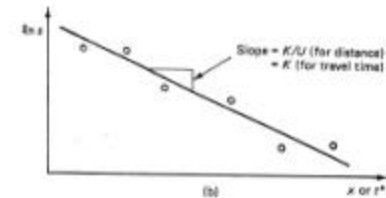
$$S = \frac{QuSu + QeSe}{Q}$$

Thomann and Mueller 1987



$$S = S_0 \exp(-kt)$$

$$S = S_0^{-kt}$$



## Example Problem

- Upstream flow is 50 cfs no background concentration of pollutant
- Discharge is 10 MGD at 100 mg/L ( $k = 0.1/\text{day}$ )
- River velocity is 5 miles/day
- What is the concentration at 10 miles downstream?
- How much reduction is needed if the 10 mi conc. must be  $< 15 \text{ mg/L}$ ?

$$Q_e = \frac{10 \times 10^6 \text{ gal}}{\text{day}} \times \frac{\text{ft}^3}{7.48 \text{ gal}} \times \frac{\text{day}}{86,400 \text{ sec}} = 15.5 \text{ ft}^3 / \text{sec}$$

$$Q = 50 + 15.5 \text{ cfs} = 65.5 \text{ cfs}$$

$$S_o = \left( \frac{15.5 \text{ cfs}}{65.5 \text{ cfs}} \right) 100 \text{ mg} / \text{L} = 23.66 \text{ mg} / \text{L}$$

$$S = S_o \exp(-kt) = (23.66 \text{ mg} / \text{L}) \exp\left(\frac{-(0.1/\text{day})(10 \text{ mi})}{5 \text{ mi} / \text{day}}\right) = 19.4 \text{ mg} / \text{L}$$

$$\left( \frac{19.4 - 15}{19.4} \right) 100 = 23\% \text{ reduction}$$

Thomann and Mueller 1987

